Automatic Digital Matric Structural Analysis

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LIST OF SYMBOLS

A =cross-sectional area of bar.

- $A_{\text{max}} = \text{maximum cross-sectional area of bar.}$
- $A_{\min} = \min m$ cross-sectional area of bar. $A_n =$ area of panel.
- $B_{\rm max}$ = length of longer parallel edge of tapered panel.
- $B_{\min} =$ length of shorter parallel edge of tapered panel.
- $D_x(i) = x$ component of displacement at the joint *i* in the coordinate system (X, Y, Z). $D_{y}(i)$ and $D_{\mathbf{z}}(i)$ are defined similarly.

E = Young's modulus.

 $F_x(i) = x$ component of the external force applied at the joint *i*. $F_{u}(i)$ and $F_{z}(i)$ are defined similarly.

G = shear modulus.

- L_{ij} = length of the bar with end joints *i* and *j*.
- $P_{ij} =$ load at the joint *i* on the bar B_{ij} .

q =shear flow.

 \vec{q} = average shear flow.

- δ_{ii} = the displacement of the joint *i* in the direction of the bar B_{ij} .
- $\Delta x_{ij} = x_i x_j$. Δy_{ij} and Δz_{ij} are defined similarly. t_D = thickness of panel at intersection of its diagonals.

i, j, k, m, n =subscripts.

INTRODUCTION

THE accelerated pace of computer development has made possible the rapid solution of complex problems. The time necessary to set up problems has begun to surpass greatly machine solution time. Consequently, more emphasis is needed on means of making use of digital machines and allied equipment in setting up problems automatically.

Heretofore, the preparation of certain matric equations appearing in structural analysis has been a tedious task. The procedures have required a large amount of judgment and tiresome hand computation. The chances for errors have been prevalent.

The present paper presents a method whereby the above factors can be minimized or negated. Input data are reduced to a minimum. All logical decisions are carried out completely automatically so as to arrange the matrix automatically. Machine time is found to be very small relative to the time previously needed to set up problems. Therefore, this coded program should prove very useful to structures and allied engineers.

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Some familiarization with Klein^{1,2} is helpful but not necessary for the understanding of the development in this paper. The basic concepts are the ones of joint, bar, and panel. The joints connect the bars and the bars border the panels.

MACHINE SIMULATION OF ELEMENTS

The code is built around the basic elements: joints, bars, and panels. The information below noted by asterisk (*) is computed.

1) Joints

A group of words is assigned to each joint containing the following information:

- a) Its position coordinates x, y, and z
- b) Whether it is fixed
- *c) All the bars attached to the joint.
- 2) Bars

A group of words is assigned to each bar with the following data:

- a) Its two end joints
- b) Whether it is tapered
- c) Its cross-sectional area (both maximum and minimum, if tapered)
- *d) The direction cosines x, y, and z
- *e) The length of the bar L_{ij} .

3) Panels

A group of words is assigned to each panel with information on:

- a) The four corner joints
- *b) The area
- c) The thickness
- *d) All of the bordering bars
- *e) B_{\max} and B_{\min} (lengths of the parallel sides)
- f) Whether it is tapered.

The number of cells in a group must be multiples of eight to allow the use of multiple index registers."A joint, bar, and panel, grouped together in 40 cells to economize on space, are not necessarily related.

The Code

A) Input

The information without asterisks, described in the previous section, is read into the proper location. The numbers of the joints appearing in the bars and panels are converted into the two's complement of that joint's address. The joint then can be referred to with an index register.

¹ B. Klein, "A simple method of matric structures analysis,"

J. Inst. Aeronaut. Sciences, vol. 24, pp. 40-46; January, 1957. ² B. Klein, "A simple method of metric structural analysis, part II—effects of taper and a consideration of curvature," J. Inst. Aeronaut. Sciences, vol. 24, pp. 813–820; November, 1957.

B) Bar Cross Referencing

1) The following is computed for each bar B_{ij} :

a)
$$\Delta x = x_i - x_j,$$
 (1)

b)
$$\Delta y = y_i - y_j,$$
 (2)

c) $\Delta z = z_i - z_j$. (3)

- The bar address complement is stored in the groups of the joints *i* and *j*.
- 3) Every panel is examined. If both the joints i and j are in it, the bar B_{ij} borders it. In this case the two's complement of the bar address is placed in the panel group and the two's complement of the panel address is placed in the bar group.

C) Joint Equations

 For each joint *i* the following force equilibrium equations are entered in the matrix

a)
$$\sum_{j} P_{ij} \left(\frac{\Delta x}{L} \right)_{ij} = F_{z}(i),$$
 (4)

b)
$$\sum_{j} P_{ij} \left(\frac{\Delta y}{L} \right) = F_{y}(i),$$
 (5)

c)
$$\sum_{j} P_{ij} \left(\frac{\Delta z}{L} \right) = F_z(i).$$
 (6)

D) Panel Area

1) The two parallel bars for each panel are found by comparing the direction cosines. The bars are rearranged so that the longest parallel side is first and the other parallel bar is second.

 Twice the area A_p is computed by the following (see Fig. 1).

$$2A_p = h(B_{\min} + B_{\max}) \tag{7}$$

where

$$h = L_{ij} \sin \theta_1 \tag{8}$$

$$\sin \theta_1 = \sqrt{1 - \cos^2 \theta_1} \tag{9}$$

$$\cos \theta_1 = \left(\frac{\Delta x}{L}\right)_{ij} \left(\frac{\Delta x}{L}\right)_{jm} + \left(\frac{\Delta y}{L}\right)_{ij} \left(\frac{\Delta y}{L}\right)_{jm} + \left(\frac{\Delta z}{L}\right)_{ij} \left(\frac{\Delta z}{L}\right)_{jm}.$$
(10)

The cosine above is equal to the dot product only if both sides are directed away from or toward the joint. If the sides are directed in opposite ways, the dot product must be multiplied by minus one.

E) Panel Equation

1) The shear panel displacement equation for the panel in Fig. 1 is

$$-(2A_p/Gt_D)\bar{q}_{jmki}-B_{\max}(\delta_{ij}+\delta_{jm})+B_{\min}(\delta_{ki}+\delta_{ik})$$

$$-L_{ij}(\delta_{ij}+\delta_{ji})+L_{km}(\delta_{km}+\delta_{mk})=0.$$
(11)





- 2) The arrows inside the panel indicate the direction of shear flow. The arrows outside the panel indicate the direction of displacements.
- The corresponding panel and bar numbering for Fig. 1 is

panel: P_{jmki}

bars: B_{mj} , B_{ki} , B_{ij} , and B_{km} .

- 4) The panel numbering determines the positive direction of the shear flows, which are directed toward the joints designated by the first and third subscripts of the panel. (See Fig. 1.) The bar numbering determines the direction of positive displacement of the joint which is toward the joint determined by the first subscript of the bar.
- 5) The signs of the terms in the equation are:
 - a) Shear term. This sign is minus.
 - b) Displacement term. If the direction of positive displacement is the same as the direction of positive shear flow along a side, this sign is plus; if they are opposite, the sign is minus.
- 6) The code determines the signs by examining the joint numbers. If the first bar subscript is equal to the first or third panel subscript, the sign is plus; otherwise, it is minus.
- 7) The length of the side is the factor outside the parenthesis in the displacement term. Note that the length of the opposite side is used in the case of the parallel sides.
- F) Bar Equations
 - 1) The axial element equilibrium equation for each bar is

$$P_{ij} - P_{ji} + \sum_{n} b_n(\pm \bar{q}_n) = 0.$$
 (12)

- 2) When the bar B_{ij} is one of the parallel bars bordering a panel, b_n of (12) is the length of the other parallel bar; otherwise, it is the length of the bar B_{ij} .
- 3) The sign of a shear term is positive if the positive direction of the shear along an edge of the

panel and the positive direction of the displacement in the bar bordering that edge are opposite; otherwise, the sign of the shear term is negative.

4) The axial force displacement equation for a tapered bar is

$$P_{ij} + L_{ij} \sum_{n} (\pm \tilde{q}_n) f(B_{\min}/B_{\max}, A_{\max}/A_{\min})$$
$$+ (E\overline{A}/L_{ij}) (\delta_{ji} - \delta_{ij}) = 0 (13)$$

where P_{ij} is the force at the narrow end of the bar. The sign of the shear term is plus if the shear and displacement directions oppose, and

$$f(B_{\min}/B_{\max}, A_{\max}/A_{\min}) = \frac{(A_{\min}/A_{\max})^{0.175}}{1 + (B_{\min}/B_{\max})^{0.7}} \quad (14)$$

$$A = A_{\min} + \left(\frac{A_{\min}/A_{\max}}{2}\right)^{0.175} (A_{\max} - A_{\min})$$
(15)

if the taper is linear.

5) For a nontapered bar the equation is

$$P_{ij} + P_{ji} + (2EA/L_{ij})(\delta_{ji} - \delta_{ij}) = 0.$$
 (16)

G) The Flow Chart follows.





Fig. 2-Example problem.

Example Problem

The structure in Fig. 2 is analyzed by the code. The complete input data describing the structure appears in the next section, which is followed by the matrix generated and the solution with the matrix column numbers. The elements without matrix column numbers are obtained by a simple calculation from matrix computed elements; *e.g.*, Delta 1–7 equals Delta 1–3.

Total computing time, *i.e.*, the time for both matric setup and solution, is about 0.01 hour.

Problems of a much more formidable nature have been arranged and solved by the program, and work is in progress for improving the code. For example, use of instantaneous coordinates may create many more zero elements in the matrix.

		INPUT	-		
FORCES					
	JOINT	X	Y	Ζ	
	3	0.0	1000.0	0.0	
	9	0.0	1000.0	0.0	
JOINTS					
ז	NUMBER	X	V	7.	FIXED
-	1	- 25	16.0	0 1	NO
	2	- 3.75	8.0	0.1	NO
	3	- 7 5	16.0	0.1	NO
	4	-11.25	8.0	0.1	NO
	5	- 5.0	0.0	0.1	YES
	6	-15.0	0.0	0.1	ŶĔŠ
	7	2.5	16.0	0.1	NO
	8	3.75	8.0	0.1	NO
	9	7.5	16.0	0.1	NO
	10	11.25	8.0	0.1	NO
	11	5.0	0.0	0.1	YES
12		15.0	0.0	0.1	YES
BARS					
JOINT	1 JOINT 2	TAPERED	AMAX	A_{MIN}	E
3	4	YES	1.0	0.66667	1.0
4	6	YES	1.3333	1.0	1.0
9 10		YES	1.0	0.66667	1.0
10 12		YES	1.3333	1.0	1.0
$\frac{1}{2}$ $\frac{2}{2}$		YES	1.0	0.66667	1.0
2 5		YES	1.3333	1.0	1.0
7 8 9 11		YES	1.0	0.00007	1.0
03	11	I LS	1.3333	1.0	1.0
3	2	NO	1.0	1.00007	1.0
0	7	NO	0.66667	0.66667	1.0
10 8		NŎ	1.0	1 0	1.0
1	ž	ŇŎ	0.66667	0.66667	1.0
$\overline{2}$	8	NO	1.0	1.0	1.0

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PANELS						
NUM- BER	JOINT	JOINT	JOINT	JOINT	G- SHEAR	THICK- NESS
$1 \\ 2$	3 4	$\frac{4}{6}$	2 5	$\frac{1}{2}$	$0.4 \\ 0.4$	0.07
	1 2	2 5	8 11	$\overline{\overline{7}}_{8}$	$0.5 \\ 0.4$	0.07 0.1
5 6	7 8	8 11	10 12	9 10	$\begin{array}{c} 0.4 \\ 0.4 \end{array}$	0.07 0.1
THE MATR	IX					
	ROW 1	C	OL	0	VALUE	
	1		23	-0	.42443388	
	$\hat{\frac{2}{2}}$		$\frac{1}{2}$	-0^{-0}_{0}	.90545894 .90545894	
	3 3		4 5	$-1 \\ 0$.0 .90545894	
	3 4		6 7	$-0 \\ 0$.42443388 .15437688	
	4 4		8 9	$-0 \\ -1 \\ 1$.15437688	
	4 5 5		7	-0^{1}	.0 .98801203	
	6		11 12	-0 -1	.15437688	
	6 7		13 11	1 0	.0 .98801203	
	8 8		14 15	$-0 \\ 0$.42443388 .42443388	
	8 9		16 14	$-1 \\ -0$.0 .90545894	
	10 10		15 17	-1^{0}	.90545894 .0	
	10 10 11		19 20	0	. 42443388	
	11 11		21 22	0 1	.15437688	
	11 12		23 20	$-1 \\ -0$.0 .98801203	
	12 13		21 24	000	.98801203 .15437688	
	13 13 14		25 26 24	-1^{1}_{0}	.0 .0 .0901202	
	15 15		24 27 28	-3499	.98601205 .99999 .8352084	
	15 15		29 30	-10^{8}	.0970674	
	15 16		31 32	$-10 \\ -3571$.0 .4285	
	16 16	:	28 4	$-8 \\ -8$.8352984 .8352984	
	16 16		29 33	$-\frac{8}{2}$.0970674 .0970674	
	10 16 16	•	54 6 30	-7	.5	
	16 17		31 35	- 3499	.0 .09999	
	17 17	-	36 29	-8	.0970674 .0970674	
	17 17		30 37	-1 1	.0 .0	
	18 18		38 36	-3571	.4285 .0970674	
	18 18 19		39 29 22	-8	.0970674	
	18		33 34 40	-7	.5	
	18 18		30 37	5	.0 .0	
	19 19		41 42	-3499 8	.9999 .8352984	
	19 19		36 37	-8 10	.0970674	
	19 20	•	43 44 42	10 - 3571	.0 .4285	
	20 20 20		±2 17 36	8 8	.0332984 .8352984 .0970674	
	20 20		39 40	-8	.0970674 .5	

ROW	COL	VALUE
20	19 37	-7.5
20	43	-5.0
21	38	5.0
21	35	-1.0
21	10	-1.0
21	23 30	-0.26666667
$\tilde{22}$	37	-0.26666667
22	10	1.0
22	23	1.0
23	38 13	-7.5 -1.0
23	26	1.0
24	34	-0.26666667
24	40	-0.26666667
24 24	15	1.0
25	32	-5.0
25	27	10.0
25	9	-1.0
25	3 30	1.0
26	31	-0.26666667
26	9	1.0
26	3	1.0
27	32 12	7.5
28	34	0.26666667
28	6	-0.266666667
28	12	1.0
29	44	5.0
29 20	41 22	-10.0 -1.0
29	16	1.0
30	37	0.26666667
30	43	-0.26666667
30	22 16	1.0
31	44	-7.5
31	25	-1.0
32	40	0.26666667
32 32	19	0.20000007
33	35	8.0970674
33	27	-8.0970674
33	45	-1.0
33 34	8 35	1.0 4.2360006
34	27	-4.2360996
34	29	-0.1430726
34	8	1.0
35	30	8.09/00/4
35	7	-1.0
35	11	1.0
36	38	4.3028433
36	33	-4.3028433 -0.10150823
36	29	0.10150823
36	11	1.0
37	41	8.0970674
37	33 46	-8.0970074 -1.0
37	21	1 .0
38	41	4.2360996
38 38	35 36	-4.2360996 -0.14207260
38	21	-0.1430/200
39	$\overline{44}$	8.0970674
39	38	-8.0970674
39 30	20	-1.0
39 40	24 44	1.0 4.3028433
40	38	-4.3028433

ROW	COL	VALUE
40	39	-0.10150823
40	36	0.10150823
40	24	1.0
41	27	8.8352984
41	-47	-1.0
41	2	1.0
42	27	4.6223159
42	28	-0.13111821
42	2	1.0
43	32	8.8352984
43	1	-1.0
44	32	4.0951448
44	4	-0.903020/3/
44	28	0.093020/3/
45	41	-8.8352984
45	4ð 15	-1.0
45	12	1.0

ROW	COL	VALUE
46	41	-4.6223159
46	42	-0.13111821
46	15	1.0
47	44	-8.8352984
47	14	-1.0
48	44	-4.6951448
48	17	-0.093026737
48	42	0.093026737
27	49	-468.75001
28	49	-468.75001
31	49	-468.75001
32	49	-468.75001
43	49	-1104.4123
44	49	-1104.4123
47	49	-1104.4123
48	49	-1104.4123

THE SOLUTION

MATRIX COLUMN NUMBER	EL	EMENT	VALUE	MATRIX COLUMN NUMBER	ELEMENT	VALUE
24	Р	1 2	0.0	35	Q 4	0.0
25	Р	1 3	1631.4448	32	Q 5	-407.4740
26	Р	1 7	1631.4448	. 27	Q 6	-105.1129
20	\mathbf{P}	2 1	3299.3447	39	DELTA 1 2	43445.283
22	Р	2 4	986.2406	40	DELTA 1 3	6117.8879
21	Р	2 5	3299.3447		DELTA 1 7	6117.8879
23	Р	2 8	986.2406	36	DELTA 2 1	26172.822
	Р	3 1	418.5939	37	DELTA 2 4	3698.4027
	Р	34	986.2406		DELTA 2 8	3898.4027
16	Р	4 2	0.0		DELTA 2 5	26172.822
14	\mathbf{P}	4 3	7443.9683		DELTA 3 1	29813.814
15	Р	4 6	7443.9683	17	DELTA 3 4	151221.70
46	Р	52	4150.4514	43	DELTA 4 2	7396.8052
48	Р	64	6515.2640	42	DELTA 4 3	53067.405
13	Р	71	1631.4448		DELTA 4 6	53067.405
11	Р	78	0.0		DELTA 7 1	6117.8879
12	Р	79	1631.4448	33	DELTA 7 8	43445.282
10	Р	82	986.2406	34	DELTA 7 9	6117.8875
7	Р	87	3299.3448		DELTA 8 2	3698.4027
	Р	8 10	986.2406	29	DELTA 8 7	26172.822
8	Р	8 11	3299.3448	30	DELTA 8 10	3698.4022
	Р	97	418.5939		DELTA 8 11	26172.822
9	Р	9 10	986.2406		DELTA 9 7	-29813.814
3	Р	10 8	0.0	4	DELTA 9 10	151221.70
1	Р	10 9	7743.9684	31	DELTA 10 8	7396.8045
2	Р	10 12	7743.9684	28	DELTA 10 9	53067.405
45	Р	11 8	4150.4514		DELTA 10 12	53067.405
47	Р	12 10	6515.2640	19	DX 3	-29813.814
44	Q	1	407.4740	18	DY 3	180986.33
41	Q	2	105.1129	6	DX 9	29813.813
38	Q	3	0.0	5	DY 9	180986.33
	-					